City of Grand Prairie

# Meeting Agenda <br> City Council Development Committee 

## Call to Order

## Agenda Items

Citizens may speak on any item on the agenda by completing and submitting a speaker card.
1 20-10466 Minutes of the September 21, 2020, City Council Development Committee meeting

2 20-10418 Proposed zone change and City of Mansfield city limit boundary realignment request
Attachments: SUNBELT LAND CITY LIMIT LINE areas in each city

3 20-10405

| 4 | $\underline{20-10468}$ | Liquor Stores Update - Presented by Bill Hills, Deputy City Manager, and Rashad <br> Jackson, Planning and Development Director |
| :--- | :--- | :--- |
| 5 | $\underline{20-10391}$ | Community Revitalization Update - Presented by Andrew Fortune, Assistant to the <br> City Manager |

## Executive Session

The City Council Development Committee may conduct a closed session pursuant to Chapter 551, Subchapter D of the Government Code, V.T.C.A. to discuss the following:

1. Section 551.071 "Consultation with Attorney"
2. Section 551.072 "Deliberation Regarding Real Property"
3. Section 551.074 "Personnel Matters"
4. Section 551.087 "Deliberations Regarding Economic Development Negotiations"

## Citizen Comments

Citizens may speak during Citizen Comments for up to five minutes on any item not on the agenda by completing and submitting a speaker card.

## Adjournment

## Certification

In accordance with Chapter 551, Subchapter C of the Government Code, V.T.C.A. the City Council Development Committee meeting agenda was prepared and posted October 9, 2020.

Mona Lisa Galicia, Deputy City Secretary

City Hall is wheelchair accessible. If you plan to attend this public meeting and you have a disability that requires special arrangements, please call Mona Lisa Galicia at 972-237-8018 at least 24 hours in advance. Reasonable accommodations will be made to assist your needs.

Legislation Details (With Text)

| File \#: | 20-10466 | Version: | Name: | Minutes of the September 21, 2020, City Council <br> Development Committee meeting |
| :--- | :--- | :--- | :--- | :--- |
| Type: | Agenda lem |  | Status: | Consent Agenda |
| File created: | $10 / 7 / 2020$ |  | In control: | City Council Development Committee |
| On agenda: | 10/13/2020 |  | Final action: |  |

## From

Monica Espinoza, Executive Assistant
Title
Minutes of the September 21, 2020, City Council Development Committee meeting

## Presenter

Jeff Wooldridge, Chairman

## Recommended Action

Approve

## Analysis



City Council Development Committee
Regular Meeting Minutes
September 21, 2020
Council Briefing Room
300 West Main Street
Grand Prairie, Texas

## Committee Members Present

Chairman Jeff Wooldridge
Councilman Mike Del Bosque
Councilman John Lopez

## Committee Members Absent

Chairman Jeff Wooldridge called a regular meeting of the City Council Development Committee to order at 5:02 p.m.

## 1. Consider CCDC minutes of the August 25, 2020 meeting.

Minutes approved.

## 2. 2021 City Council Development Committee Meeting Schedule

Chairman Jeff Woolridge opened discussion of meeting schedule. Deputy City Manager Bill Helm stated dates are tentative and are needed to draft council calendar.
3. Accessory Structures - Review and discuss regulations for accessory structures, focusing on size, flexibility on required materials and administrative variances. Review of proposed Unified Development Code amendment language.

Planning Director, Rashad Jackson, discussed the introduction of revisions to the Unified Development Code (UDC) for the building materials, size, and height for accessory structures. Mr. Jackson gave an overview of examples from other benchmark cities where a combination of regulations for height and size vary based on the existing neighborhoods and past variance requests. He stated the request for a maximum of 22 feet as a revision for the building height of accessory structures. Mr. Jackson also referenced the size of accessory structures and detached garages limitations and requested that the maximum square feet
stipulation of 750 square feet be removed and for the maximum square feet be $50 \%$ of the principal dwelling based on the zoning district. Additionally, he stated concerns regarding the maximum wall height for small structures of less than 200 square feet. The maximum height will be 10 feet based on the underlying zoning district, as measured from the finished floor to the roof plate. He stated that this will limit how tall the roof can be taking into consideration the roof pitch requirements already in place. Therefore, this would not necessarily create another hardship for residents. Mr. Jackson also stated to remove the specific square footage requirement for the size of accessory structure, as long as, the structure does not exceed the maximum lot coverage and size of the principal structure, which is already stated in the UDC.

Chairman Del Bosque asked someone with five (5) acres with a 10,000 square foot home and wanted to build a detached garage, a workshop, and a barn what the size requirements would be based on the size of the home. Mr. Jackson clarified that size requirement would be based on the size of the lot, not the principal home, which is based on the underlying zoning.

Mr. Jackson stated the request for a building material exception per House Bill 2439 (HB2439) and its regulations. He stated the exception request for the principal home is addressed in the UDC already but that a building material requirement cannot be upheld for accessory structures based on HB2439. He stated we can only enforce architectural details like articulations. He proposed to allow for metal to be used as an exterior building material of accessory structures larger than 200 square feet that adhere to the building code. Additionally, the structure must be located 30 feet from the front property line. Mr. Jackson added that neighborhoods with larger accessory structures have them placed at the far end of the lot. He also stated that one caveat for those neighborhoods a part of a HOAs staff would not want to approve anything that would conflict with the existing HOA requirements or restrictions.

Chairman Woolridge stated that applicants should confirm if proposed structure is allowed in their HOA even if city staff or committee say it is fine. Mr. Jackson agreed. He also stated that these revisions would decrease the number of variance applications for accessory structures.

Chairman Del Bosque what the revisions are applicable, or it is only for special situations.

## 4. Hybrid Housing - Discuss Hybrid Housing product, design preferences and possible regulations.

Planning Director, Rashad Jackson, introduced the concept of Hybrid Housing and using the standards of the town house zoning to address concerns of multi-family and single-family housing. He stated that using the town home development standards would help contribute to a true mix of hybrid housing design. Mr. Jackson discussed issues related to the Avila project regarding building orientation along the frontage and right-of-way screening, he recommended that the screening be at least up to the eve of the abutting home or setbacks further from the right-of-way. He also discussed the need for additional parking and/or direct to access parking homes as the current parking is designed for multi-family and not for the mix of both. Additionally, he stated the building materials that face the right-of-way could be addressed by requiring a mixture of materials
on all sides of the façade. He also discussed more open space that are conducive to the overall layout of the townhomes and single-family housing types along with adding interior drives to provide direct access to garages. He stated that staff recommended building orientation to face the right-of-way and allow some exceptions to face the sides. Mr. Jackson further mentioned that the discussed recommendations already exist in the Unified Development Code (UDC) but are under the town home standards and they should also be used for the proposed Hybrid Housing.

Chairman Del Bosque about restriction of materials. Mr. Jackson stated we cannot at this time due to the current House Bill (HB2439). Chairman Wooldridge also asked if this would be overturned soon.

Chairman Del Bosque additionally asked about exceptions to the material requirement and if they can be denied based on the materials used. Mr. Jackson stated they could not be denied solely due to materials, but we could encourage masonry materials.

Chairman Lopez asked where the proposed language would be located in the UDC. Mr. Jackson stated there isn't an official location of the language yet.

Chairman Woolridge also asked Mr. Jackson if there can be other wording for townhomes instead of "For Rent" Hybrid Housing choices. Mr. Jackson stated as part of the amendment proposed that a definition of Hybrid Housing be included.

## Other Business:

Chairman Del Bosque asked about certain items and which committee should discuss certain items.
With no other business, the meeting was adjourned at 5:45 p.m.

[^0]Legislation Details (With Text)

| File \#: | 20-10418 | Version: 1 | Name: | Proposed zone change and City of Mansfield city limit request |
| :---: | :---: | :---: | :---: | :---: |
| Type: | Agenda Item |  | Status: | Agenda Ready - Committee |
| File created: | 9/28/2020 |  | In control: | City Council Development Committee |
| On agenda: | 10/13/2020 |  | Final action: |  |
| Title: | Proposed zone change and City of Mansfield city limit boundary realignment request |  |  |  |
| Sponsors: |  |  |  |  |
| Indexes: |  |  |  |  |
| Code sections: |  |  |  |  |
| Attachments: | SUNBELT LAND CITY LIMIT LINE |  |  |  |
| areas in each city |  |  |  |  |
| Date | Ver. Action By |  |  | n Result |

## From

Rashad Jackson, AICP - Planning \& Development Director

## Title

Proposed zone change and City of Mansfield city limit boundary realignment request

## Presenter

Rashad Jackson, AICP - Planning \& Development Director

## Recommended Action

Approve

## Analysis

Staff was contacted by developer, Walter Nelson, in regards to a proposed residential project in South Grand Prairie. The property is located along Highway 360 south of Lone Star Drive. The property is bisected by City of Mansfield and City of Grand Prairie city limits. The applicant has plans to rezone the property to allow for single family development. He has also noted the City of Mansfield would be willing to give their portion of the subject property to the City of Grand Prairie. The request is being reviewed to discuss the proposed rezoning and possible city limit boundary realignment.

## Financial Consideration

N/A



Legislation Details (With Text)


From
Rashad Jackson

Title
Proposed Master Thoroughfare Plan Amendment and associated development review of Cottages at Dechman.

## Presenter

Rashad Jackson, Planning and Development Director

## Recommended Action

Review and Provide Direction

## Analysis

The Cottages at Dechman is a proposed hybrid housing development located at the northwest corner of Dechman Drive and the IH - 20 frontage road. The project will consist of 150 one-and two-bedroom rental homes. Access to the property will be via a full-access driveway to Dechman Drive. The City of Grand Prairie's 2015 Transportation Thoroughfare Plan shows a collector roadway between Dechman Drive and the IH - 20 westbound frontage road that would run through the rear of this property. The proposed site plan does not show this collector. The applicant has proposed a revision to the thoroughfare plan to remove this proposed collector.

The City of Grand Prairie requires analysis of any Thoroughfare Plan amendment by the CCDC. The proposed amendment would eliminate a collector roadway directly north of the subject property. The review will
examine the effects of the proposed hybrid housing development and thoroughfare revision on Dechman Drive.

## Department of Transportation Analysis - Brett Huntsman, City of Grand Prairie Transportation Planner

The Master Thoroughfare Plan (MTP) identifies an unnamed Collector which creates an additional connection from Fish Creek Rd./ Dechman Dr. to the Interstate 20 WB Frontage Road. This segment is located along the border of the Dallas County floodway and a vacant lot within PD-20.

The Collector is being addressed due to developer interest in the vacant lot. The roadway is not necessary for the site's operation and, therefore, is not being considered for construction by the developer. Moreover, the developer is not wishing to dedicate the required 70 ' right-of-way necessary to build the roadway.

City staff recommends removing this section from the MTP.

As required by the Unified Development Code, the developer of the vacant lot has provided a traffic impact analysis (TIA) identifying operations of the site and nearby intersections at buildout of the property. The TIA found that there would be no significant impact to the existing roadways and intersections following construction of the new development without the unnamed Collector.

Data was collected for the TIA included detoured traffic from the ongoing l-20 frontage road construction. Once the intersections of the I-20 Frontage and Carrier Parkway are completed, traffic volumes are anticipated to be reduced.

The recommendation of the TIA is that the roadway be removed from the MTP and that the project moves forward. City staff agrees with the recommendation.

Staff does not believe that the unnamed Collector would create additional development potential in the area. The parcel on the opposite side of the proposed development is Dallas County-owned floodway. There would be a considerable cost to acquire and develop this land with potentially significant impact to the existing floodway.

325
325



## Traffic Impact Analysis

## Cottages at Dechman <br> Grand Prairie, Texas

28 September 2020


#  <br> GROUPB, LLC 

## Cottages at Dechman - Grand Prairie

I hereby certify that this report was prepared by me or under my direct supervision, and that I am a duly Licensed Professional Engineer under the laws of the State of Texas.


Scott P. Israelson, P.E., PTOE
License No. 116712

## Executive Summary

## Project Description

The Cottages at Dechman is a proposed development in Grand Prairie, Texas. The project will consist of 150 one- and two-bedroom rental casitas. The site is located in the northwest quadrant of $\mathrm{IH}-20$ \& Dechman Drive.

Access to the property will be via a full-access driveway to Dechman Drive. The City of Grand Prairie's 2015 Transportation Thoroughfare Plan shows a collector roadway between Dechman Drive and the IH20 westbound frontage road that would run through this property, but the site plan does not include this collector.

The City of Grand Prairie requires this analysis for the Thoroughfare Plan amendment that would eliminate the collector roadway, and to examine the effects of development on Dechman Drive.

## Trip Generation

The proposed new development is expected to generate 1,510 daily trips, with 28 entering trips and 83 exiting trips in the AM peak hour, and 95 entering and 55 exiting trips in the PM peak hour.

## Turn Lanes

Analysis shows that left-turn volumes are projected to meet thresholds for a turn lane. It is recommended to construct a northbound left-turn lane.

## Traffic Impacts

Analysis shows that the intersections in the study area are projected to continue to operate acceptably without the proposed connector between Dechman Drive and the $\mathrm{IH}-20$ westbound frontage road. It is recommended to seek a Thoroughfare Plan amendment that allows the development to proceed without constructing the connector between Dechman Drive and the IH-20 westbound frontage road.

## 

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##  <br> GROUP, LLC

## I. Introduction

The Cottages at Dechman is a proposed residential development in Grand Prairie, Texas. The site is located in the northwest quadrant of $\mathrm{IH}-20 \&$ Dechman Drive.

The project will consist of 150 one- and two-bedroom rental casitas. Access to the property will be via a full-access driveway to Dechman Drive. The City of Grand Prairie's 2015 Transportation Thoroughfare Plan shows a collector roadway between Dechman Drive and the $\mathrm{IH}-20$ westbound frontage road that would run through this property, although this is not shown on the site plan.

The City of Grand Prairie requires this analysis for the Thoroughfare Plan amendment, and to examine the effects of development on Dechman Drive.

The study area included the following intersections:

- Dechman Drive \& Project Access
- Dechman Drive \& IH-20 westbound frontage road
- Dechman Drive \& IH-20 eastbound frontage road

The study analyzed the following scenarios:

- 2020 Existing Conditions
- Full Build 2021 Conditions

The AM peak hour and PM peak hour were analyzed.
Figure 1 shows the most recent site plan. Figure 2 shows the project vicinity map.


Dechman Drive \& Project Access - looking south


Site Plan
Figure 1
Date: 14 September 2020

Cottages at Dechman - Grand Prairie


## II. Existing Conditions

## A. Existing Roadway Conditions

Dechman Drive is a two-lane roadway with a posted speed limit of 35 mph .
The $\mathrm{IH}-20$ frontage roads are two-lane one-way roadways with a posted speed limit of 45 mph .
The City's 2015 Transportation Thoroughfare Plan shows a collector connection between Dechman Drive and the $\mathrm{IH}-20$ westbound frontage road that would run through this property.

## B. Existing Intersection Geometry

The proposed property connection to Dechman Drive will be a full-access driveway located at the Prairie Gate Community Apartments driveway. Dechman Drive has a southbound left-turn lane.

Dechman Drive \& IH-20 westbound frontage road is controlled with portable temporary signals. The westbound approach consists of a left-turn lane, two through lanes, and a right-turn lane. The northbound approach has a left-turn lane and one through lane. The southbound approach has one through lane and a shared through-right lane.

Dechman Drive \& IH-20 eastbound frontage road is also controlled with portable temporary signals. The eastbound approach consists of a left-turn lane, two through lanes, and a rightturn lane. The southbound approach has a left-turn lane and one through lane. The northbound approach has one through lane and a shared through-right lane.

The geometric configuration of all intersections in the study area is shown in Figure 3.

## C. Traffic Volumes

Traffic data collection for study area intersections was performed on August 25, 2020. Figure 4 displays existing traffic volumes. These volumes can be found in the Appendix.

Current traffic patterns may have been affected by "social distancing". This analysis examines historical traffic counts and compares to current data. Dividing historical data by current data determines the appropriate factor to apply in order to estimate "existing" traffic volumes.

According to the TxDOT planning office website, the daily traffic on Dechman Drive north of IH20 in 2019 was 3,549 vehicles per day.

In August 2020, the daily traffic count on Dechman Drive at the same spot is $3,821 \mathrm{vpd}$.
Since 2020 traffic volumes are higher than historical counts, the analysis uses current data.


Study Area Roadways and Intersections
Figure 3
Date: 14 September 2020

Cottages at Dechman - Grand Prairie


Existing Traffic Volumes
Figure 4
Date: 14 September 2020

Cottages at Dechman - Grand Prairie

## III. Methodology

## A. Base Assumptions

Intersection capacity analysis was conducted using Synchro v10.0. Trip generation was calculated using the 10th edition of the Institute of Transportation Engineers (ITE) Trip Generation Manual. Right-turn and left-turn lanes were examined using the National Cooperative Highway Research Program (NCHRP) Report No. 279 Intersection Channelization Design Guide.

## B. Background Growth

The City of Grand Prairie required a 3\% per year background growth for this analysis.

## C. Trip Generation

The development is proposed to consist of 150 single family homes. The ITE Trip Generation Manual, 10th Edition was used to estimate the projected trips by this development. Table 3.2 contains the summary of the land uses and sizes used for trip generation estimates.

| Table 3.2-ITE Trip Generation |  |  |  |  |  |  |  |  |
| :---: | ---: | ---: | :--- | ---: | ---: | ---: | ---: | ---: |
| Average Weekday Driveway Volumes | AM Peak <br> Hour |  | PM Peak <br> Hour |  |  |  |  |  |
| Land Use | ITE <br> Code | Size |  | Daily <br> Trips | Enter | Exit | Enter | Exit |
| Single-Family Detached <br> Housing | 210 | 150 | Dwelling Units | 1510 | 28 | 83 | 95 | 55 |

## D. Trip Distribution

Trips for this proposed development were assigned to the surrounding roadway network based on existing traffic patterns. The proposed trip distribution for this project is:

- $5 \%$ to/from the north on Dechman Drive,
- $60 \%$ to/from the west on IH-20
- $35 \%$ to/from the east on IH-20

The projected site trips are shown in Figure 5 and Full Build 2021 volumes are shown in Figure 6.



Full Build 2021 Volumes
Figure 6
Date: 28 September 2020

Cottages at Dechman - Grand Prairie

## IV. Turn Lane/Access Management

## A. Right-Turn Lanes

The National Cooperative Highway Research Program (NCHRP) Report 279 Intersection Channelization Design Guide was used to determine right-turn lane and left-turn lane thresholds for this study.

For public officials that do not have formal thresholds for determining when new access requires turn lane treatments, the NCHRP Report 279 is a tool in assessing the impacts from development. Specifically, this report allows the traffic engineering professional to input roadway type, posted speed, advancing volume (and opposing volume for left turns), and number of turning vehicles. The result is a plot on a graph defined by the above inputs recommending turn lanes or not.

Table 4.1 shows the volumes used for analysis.

| Table 4.1 - Right-Turn Lane Analysis |  |  |  |  |  |  |
| :---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Driveway | AM <br> PM | Approach | Posted <br> Speed | Advancing <br> Vol | RT <br> Vol | Turn <br> Lane <br> needed? |
|  <br> Project Access | AM | SB | 35 | 68 | 1 | No |
|  | PM |  | 175 | 5 | No |  |

Based on Full Build 2021 volumes, no turn lanes from Dechman Drive are required for the project driveway. These calculations can be found in the Appendix.
B. Left-Turn Lanes

Table 4.2 shows the volumes used in the analysis.

| Table 4.2 - Left-Turn Lane Analysis |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Driveway | $\begin{aligned} & \text { AM } \\ & \text { PM } \end{aligned}$ | Approach | Posted <br> Speed | Advancing Vol | Opposing Vol | $\begin{aligned} & \text { LT } \\ & \text { Vol } \end{aligned}$ | Turn Lane needed? |
| Dechman Dr \& Project Access | AM | NB | 35 | 80 | 68 | 27 | Yes |
|  | PM |  |  | 113 | 175 | 90 | Yes |

Based on Full Build 2021 volumes, a left-turn lane would be needed at the project driveway. These calculations can be found in the Appendix.

## 

## V. Capacity Analysis

The Transportation Research Board's Highway Capacity Manual (HCM) utilizes a term "level of service" (LOS) to measure how traffic operates in intersections. There are currently six levels of service ranging from A to F. Level of Service "A" represents the best conditions and Level of Service "F" represents the worst. Synchro software was used to determine the level of service for intersections in the study area. All worksheet reports from the analyses can be found in the Appendix.

Table 5.1 shows the control delay per vehicle associated with LOS A through $F$ for signalized and unsignalized intersections.

Table 5.1 - Highway Capacity Manual Levels of Service and Control Delay

| Signalized Intersection |  | Unsignalized Intersection |  |
| :---: | :---: | :---: | :---: |
| Level of Service | Control Delay per <br> Vehicle (sec) | Level of <br> Service | Control Delay per <br> Vehicle <br> (sec) |
| A | $\leq 10$ | A | $\leq 10$ |
| B | $>10$ and $\leq 20$ | B | $>10$ and $\leq 15$ |
| C | $>20$ and $\leq 35$ | C | $>15$ and $\leq 25$ |
| D | $>35$ and $\leq 55$ | D | $>25$ and $\leq 35$ |
| E | $>55$ and $\leq 80$ | E | $>35$ and $\leq 50$ |
| F | $>80$ | F | $>50$ |

##  <br> GROUP, LLC

## A. Dechman Drive \& Access

The proposed property connection to Dechman Drive will be a full-access driveway located at the Prairie Gate Community Apartments driveway. Dechman Drive has a southbound left-turn lane.

Table 5.2 shows the current LOS, control delay, and 95th percentile queue length for existing conditions.

| Table 5.2-Intersection LOS, Delay, and Queue by Movement - 2020 Existing |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | Approach | Movement | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& access | WB | LT | B | 10.0 | 8' | B | 11.6 | 10' |
|  |  | RT |  |  |  |  |  |  |
|  | NB | TH | Free |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |  |  |  |  |  |
|  | SB | LT | A | 7.4 | - | A | 7.4 | - |
|  |  | TH | Free |  |  |  |  |  |

Table 5.3 shows the expected LOS, control delay, and 95th percentile queue length for Full Build 2021 conditions.

| Intersection | Approach | Movement | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& access | EB | LT | A | 9.2 | 10' | B | 10.2 | 8' |
|  |  | TH |  |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |
|  | WB | LT | B | 12.2 | 13 " | C | 18.6 | 18' |
|  |  | TH |  |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |
|  | NB | LT | A | 7.4 | - | A | 7.9 | 8' |
|  |  | TH | Free |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |  |  |  |  |  |
|  | SB | LT | A | 7.4 | - | A | 7.7 | - |
|  |  | TH | Free |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |  |  |  |  |  |

## 

The City requested level-of-service analysis for Dechman Drive without the northbound leftturn lane. Table 5.3B shows the expected LOS, control delay, and 95th percentile queue length for Full Build 2021 conditions without the left-turn lane.

| Intersection | Approach | Movement | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& access | EB | LT | A | 9.2 | 10' | B | 10.2 | 8' |
|  |  | TH |  |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |
|  | WB | LT | B | 12.2 | 13 " | C | 18.8 | $20^{\prime}$ |
|  |  | TH |  |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |
|  | NB | LT | Free |  |  |  |  |  |
|  |  | TH |  |  |  |  |  |  |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |  |  |  |  |  |
|  | SB | LT | A | 7.4 | - | A | 7.7 | - |
|  |  | TH | Free |  |  |  |  |  |
|  |  | RT |  |  |  |  |  |  |  |  |  |  |  |

Analysis shows that the westbound driveway would see slightly worse conditions without a northbound left-turn lane. This shows that the turn lane has a negligible effect on traffic operations and would mostly be a safety improvement.

##  <br> GROUP, LLC

## B. Dechman Drive \& IH-20 WB frontage road

Dechman Drive \& IH-20 westbound frontage road is controlled with portable temporary signals. The westbound approach consists of a left-turn lane, two through lanes, and a right-turn lane. The northbound approach has a left-turn lane and one through lane. The southbound approach has one through lane and a shared through-right lane.

Table 5.4 shows the current LOS, control delay, and 95th percentile queue length for existing conditions.

| Intersection | Approach | Movement | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& IH-20 WB frontage rd | WB | LT | A | 7.0 | 41' | B | 10.2 | 134' |
|  |  | TH | A | 6.6 | 31 ' | A | 7.5 | 60' |
|  |  | RT | A | 0.6 | - | A | 1.8 | - |
|  | NB | LT | B | 14.5 | $30^{\prime}$ | B | 18.0 | 55' |
|  |  | TH | B | 14.1 | 42' | B | 17.3 | 93' |
|  | SB | TH | A | 9.5 | 21' | B | 10.4 | 42' |
|  |  | RT |  |  |  |  |  |  |
|  | OVERALL |  | A (8.6) |  |  | B (10.6) |  |  |

Table 5.5 shows the expected LOS, control delay, and 95th percentile queue length for Full Build 2021 conditions.

| Intersection | Approach | Movement | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& IH-20 WB frontage rd | WB | LT | A | 7.1 | 42' | B | 11.0 | 154' |
|  |  | TH | A | 6.6 | 32' | A | 8.1 | 69' |
|  |  | RT | A | 1.3 | - | A | 2.7 | 16 ' |
|  | NB | LT | B | 15.0 | $31{ }^{\prime}$ | B | 18.5 | 60' |
|  |  | TH | B | 14.6 | 50' | B | 19.2 | 129' |
|  | SB | TH | A | 8.3 | $30^{\prime}$ | A | 9.7 | $50^{\prime}$ |
|  |  | RT |  |  |  |  |  |  |
|  | OVERALL |  | A (8.5) |  |  | B (11.2) |  |  |

##  <br> GROUP, LLC

## C. Dechman Drive \& IH-20 EB frontage road

Dechman Drive \& IH-20 eastbound frontage road is also controlled with portable temporary signals. The eastbound approach consists of a left-turn lane, two through lanes, and a rightturn lane. The southbound approach has a left-turn lane and one through lane. The northbound approach has one through lane and a shared through-right lane.

Table 5.6 shows the current LOS, control delay, and 95th percentile queue length for existing conditions.

| Intersection | Approach | Movement |  | AM |  |  | PM |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& IH-20 EB frontage rd | EB | LT | A | 7.4 | $37^{\prime}$ | B | 14.2 | $90^{\prime}$ |
|  |  | TH | A | 7.4 | 45 | B | 13.4 | 96' |
|  |  | RT | A | 1.9 | 7' | A | 4.7 | 22' |
|  | NB | TH | A | 4.0 | 28 | A | 5.4 | 33' |
|  |  | RT |  |  |  |  |  |  |
|  | SB | LT | B | 13.2 | 31 ' | A | 9.7 | 15' |
|  |  | TH | B | 16.8 | 86' | C | 20.9 | $215{ }^{\prime}$ |
|  | OVERALL |  | A (7.8) |  |  | B (13.9) |  |  |

Table 5.7 shows the expected LOS, control delay, and 95th percentile queue length for Full Build 2021 conditions.

| Intersection | Approach | Movement | AM |  |  | PM |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | LOS | Delay | Queue | LOS | Delay | Queue |
| Dechman Dr \& IH-20 EB frontage rd | EB | LT | A | 7.8 | $37^{\prime}$ | B | 15.5 | 123' |
|  |  | TH | A | 7.7 | 46' | B | 13.7 | 99' |
|  |  | RT | A | 2.1 | $7{ }^{\prime}$ | A | 4.7 | 22' |
|  | NB | TH | A | 4.4 | 31 | A | 5.7 | $36^{\prime}$ |
|  |  | RT |  |  |  |  |  |  |
|  | SB | LT | B | 15.3 | 31 ' | B | 10.3 | $24^{\prime}$ |
|  |  | TH | B | 16.7 | 87' | C | 21.1 | 223' |
|  | OVERALL |  | A (8.3) |  |  | B (14.3) |  |  |

Analysis shows that the intersections in the study area are projected to continue to operate acceptably without the proposed connector between Dechman Drive and the IH-20 westbound frontage road. It is recommended to seek a Thoroughfare Plan amendment.

## VI. Summary and Conclusion

This study serves as an analysis of the traffic impacts from the Cottages at Dechman development in Grand Prairie, Texas.

This analysis was necessary due to the elimination of a connector roadway between Dechman Drive and the IH-20 westbound frontage road which would run through the property. Eliminating this roadway requires a Thoroughfare Plan amendment.

## Trip Generation

The proposed new development is expected to generate 1,510 daily trips, with 28 entering trips and 83 exiting trips in the AM peak hour, and 95 entering and 55 exiting trips in the PM peak hour.

## Turn Lanes

Analysis shows that left-turn volumes are projected to meet thresholds for a turn lane. It is recommended to construct a northbound left-turn lane.

## Traffic Impacts

Analysis shows that the intersections in the study area are projected to continue to operate acceptably without the proposed connector between Dechman Drive and the IH-20 westbound frontage road. It is recommended to seek a Thoroughfare Plan amendment that allows the development to proceed without constructing the connector between Dechman Drive and the $\mathrm{IH}-20$ westbound frontage road.


Dechman Drive \& Access - looking north

#  

## Appendix

Background Information

Traffic Volumes
Trip Generation
Capacity Analysis
Turn Lanes

## BACKGROUND INFORMATION

AADT and AADT Trucks by Year for 1/1/2010-12/31/2019

```
District Dallas
Location ID 57HP6671
County Dallas Located On Dechman Dr
Community Grand Prairie
LRS ID
LRS Point
Located On Dechman Dr
LRS ID
```

AADT AND AADT TRUCK TREND


## TRAFFIC VOLUMES

|  | A | B | C | D | E |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | DECHMAN DR NORTH OF IH-20 |  |  |  |  |
| 2 |  |  |  |  |  |
| 3 | Start Date: 8/25/2020 |  |  |  |  |
| 4 | Start Time: 12:00:00 AM |  |  |  |  |
| 5 | Site Code: 960 |  |  |  |  |
| 6 |  |  |  |  |  |
| 7 | Date | Time | NB | SB | TOTAL |
| 8 | 8/25/2020 | 12:00 AM | 5 | 4 | 9 |
| 9 | 8/25/2020 | 12:15 AM | 5 | 3 | 8 |
| 10 | 8/25/2020 | 12:30 AM | 3 | 0 | 3 |
| 11 | 8/25/2020 | 12:45 AM | 1 | 2 | 3 |
| 12 | 8/25/2020 | 01:00 AM | 4 | 0 | 4 |
| 13 | 8/25/2020 | 01:15 AM | 3 | 0 | 3 |
| 14 | 8/25/2020 | 01:30 AM | 0 | 1 | 1 |
| 15 | 8/25/2020 | 01:45 AM | 2 | 0 | 2 |
| 16 | 8/25/2020 | 02:00 AM | 2 | 1 | 3 |
| 17 | 8/25/2020 | 02:15 AM | 2 | 3 | 5 |
| 18 | 8/25/2020 | 02:30 AM | 1 | 3 | 4 |
| 19 | 8/25/2020 | 02:45 AM | 1 | 4 | 5 |
| 20 | 8/25/2020 | 03:00 AM | 1 | 2 | 3 |
| 21 | 8/25/2020 | 03:15 AM | 1 | 1 | 2 |
| 22 | 8/25/2020 | 03:30 AM | 2 | 0 | 2 |
| 23 | 8/25/2020 | 03:45 AM | 0 | 1 | 1 |
| 24 | 8/25/2020 | 04:00 AM | 1 | 4 | 5 |
| 25 | 8/25/2020 | 04:15 AM | 1 | 3 | 4 |
| 26 | 8/25/2020 | 04:30 AM | 3 | 4 | 7 |
| 27 | 8/25/2020 | 04:45 AM | 0 | 5 | 5 |
| 28 | 8/25/2020 | 05:00 AM | 0 | 3 | 3 |
| 29 | 8/25/2020 | 05:15 AM | 1 | 6 | 7 |
| 30 | 8/25/2020 | 05:30 AM | 2 | 10 | 12 |
| 31 | 8/25/2020 | 05:45 AM | 6 | 6 | 12 |
| 32 | 8/25/2020 | 06:00 AM | , | 9 | 13 |
| 33 | 8/25/2020 | 06:15 AM | 5 | 15 | 20 |
| 34 | 8/25/2020 | 06:30 AM | 7 | 15 | 22 |
| 35 | 8/25/2020 | 06:45 AM | 9 | 20 | 29 |
| 36 | 8/25/2020 | 07:00 AM | 14 | 34 | 48 |
| 37 | 8/25/2020 | 07:15 AM | 33 | 37 | 70 |
| 38 | 8/25/2020 | 07:30 AM | 27 | 34 | 61 |
| 39 | 8/25/2020 | 07:45 AM | 18 | 24 | 42 |
| 40 | 8/25/2020 | 08:00 AM | 15 | 18 | 33 |
| 41 | 8/25/2020 | 08:15 AM | 23 | 16 | 39 |
| 42 | 8/25/2020 | 08:30 AM | 20 | 24 | 44 |
| 43 | 8/25/2020 | 08:45 AM | 22 | 24 | 46 |
| 44 | 8/25/2020 | 09:00 AM | 10 | 22 | 32 |
| 45 | 8/25/2020 | 09:15 AM | 18 | 20 | 38 |
| 46 | 8/25/2020 | 09:30 AM | 23 | 20 | 43 |
| 47 | 8/25/2020 | 09:45 AM | 20 | 24 | 44 |
| 48 | 8/25/2020 | 10:00 AM | 23 | 24 | 47 |
| 49 | 8/25/2020 | 10:15 AM | 16 | 21 | 37 |
| 50 | 8/25/2020 | 10:30 AM | 21 | 28 | 49 |
| 51 | 8/25/2020 | 10:45 AM | 21 | 26 | 47 |
| 52 | 8/25/2020 | 11:00 AM | 22 | 24 | 46 |
| 53 | 8/25/2020 | 11:15 AM | 24 | 18 | 42 |


|  | A | B | C | D | E |
| ---: | :---: | :---: | ---: | ---: | ---: |
| 54 | $8 / 25 / 2020$ | $11: 30 \mathrm{AM}$ | 28 | 38 | 66 |
| 55 | $8 / 25 / 2020$ | $11: 45 \mathrm{AM}$ | 30 | 32 | 62 |
| 56 | $8 / 25 / 2020$ | $12: 00 \mathrm{PM}$ | 24 | 24 | 48 |
| 57 | $8 / 25 / 2020$ | $12: 15 \mathrm{PM}$ | 30 | 28 | 58 |
| 58 | $8 / 25 / 2020$ | $12: 30 \mathrm{PM}$ | 39 | 34 | 73 |
| 59 | $8 / 25 / 2020$ | $12: 45 \mathrm{PM}$ | 38 | 19 | 57 |
| 60 | $8 / 25 / 2020$ | $01: 00 \mathrm{PM}$ | 27 | 30 | 57 |
| 61 | $8 / 25 / 2020$ | $01: 15 \mathrm{PM}$ | 32 | 27 | 59 |
| 62 | $8 / 25 / 2020$ | $01: 30 \mathrm{PM}$ | 30 | 34 | 64 |
| 63 | $8 / 25 / 2020$ | $01: 45 \mathrm{PM}$ | 26 | 20 | 46 |
| 64 | $8 / 25 / 2020$ | $02: 00 \mathrm{PM}$ | 29 | 25 | 54 |
| 65 | $8 / 25 / 2020$ | $02: 15 \mathrm{PM}$ | 22 | 20 | 42 |
| 66 | $8 / 25 / 2020$ | $02: 30 \mathrm{PM}$ | 36 | 25 | 61 |
| 67 | $8 / 25 / 2020$ | $02: 45 \mathrm{PM}$ | 30 | 27 | 57 |
| 68 | $8 / 25 / 2020$ | $03: 00 \mathrm{PM}$ | 36 | 27 | 63 |
| 69 | $8 / 25 / 2020$ | $03: 15 \mathrm{PM}$ | 30 | 36 | 66 |
| 70 | $8 / 25 / 2020$ | $03: 30 \mathrm{PM}$ | 35 | 34 | 69 |
| 71 | $8 / 25 / 2020$ | $03: 45 \mathrm{PM}$ | 33 | 32 | 65 |
| 72 | $8 / 25 / 2020$ | $04: 00 \mathrm{PM}$ | 33 | 44 | 77 |
| 73 | $8 / 25 / 2020$ | $04: 15 \mathrm{PM}$ | 34 | 26 | 60 |
| 74 | $8 / 25 / 2020$ | $04: 30 \mathrm{PM}$ | 43 | 50 | 93 |
| 75 | $8 / 25 / 2020$ | $04: 45 \mathrm{PM}$ | 52 | 58 | 110 |
| 76 | $8 / 25 / 2020$ | $05: 00 \mathrm{PM}$ | 40 | 60 | 100 |
| 77 | $8 / 25 / 2020$ | $05: 15 \mathrm{PM}$ | 50 | 52 | 102 |
| 78 | $8 / 25 / 2020$ | $05: 30 \mathrm{PM}$ | 42 | 40 | 82 |
| 79 | $8 / 25 / 2020$ | $05: 45 \mathrm{PM}$ | 49 | 35 | 84 |
| 80 | $8 / 25 / 2020$ | $06: 00 \mathrm{PM}$ | 42 | 34 | 76 |
| 81 | $8 / 25 / 2020$ | $06: 15 \mathrm{PM}$ | 32 | 38 | 70 |
| 82 | $8 / 25 / 2020$ | $06: 30 \mathrm{PM}$ | 36 | 28 | 64 |
| 83 | $8 / 25 / 2020$ | $06: 45 \mathrm{PM}$ | 39 | 42 | 81 |
| 84 | $8 / 25 / 2020$ | $07: 00 \mathrm{PM}$ | 38 | 34 | 72 |
| 85 | $8 / 25 / 2020$ | $07: 15 \mathrm{PM}$ | 30 | 34 | 64 |
| 86 | $8 / 25 / 2020$ | $07: 30 \mathrm{PM}$ | 38 | 40 | 78 |
| 87 | $8 / 25 / 2020$ | $07: 45 \mathrm{PM}$ | 31 | 26 | 57 |
| 88 | $8 / 25 / 2020$ | $08: 00 \mathrm{PM}$ | 34 | 28 | 62 |
| 89 | $8 / 25 / 2020$ | $08: 15 \mathrm{PM}$ | 38 | 30 | 68 |
| 90 | $8 / 25 / 2020$ | $08: 30 \mathrm{PM}$ | 32 | 18 | 50 |
| 91 | $8 / 25 / 2020$ | $08: 45 \mathrm{PM}$ | 30 | 18 | 48 |
| 92 | $8 / 25 / 2020$ | $09: 00 \mathrm{PM}$ | 30 | 20 | 50 |
| 93 | $8 / 25 / 2020$ | $09: 15 \mathrm{PM}$ | 14 | 7 | 21 |
| 94 | $8 / 25 / 2020$ | $09: 30 \mathrm{PM}$ | 14 | 18 | 32 |
| 95 | $8 / 25 / 2020$ | $09: 45 \mathrm{PM}$ | 12 | 14 | 26 |
| 96 | $8 / 25 / 2020$ | $10: 00 \mathrm{PM}$ | 18 | 12 | 30 |
| 97 | $8 / 25 / 2020$ | $10: 15 \mathrm{PM}$ | 16 | 4 | 20 |
| 98 | $8 / 25 / 2020$ | $10: 30 \mathrm{PM}$ | 8 | 11 | 19 |
| 99 | $8 / 25 / 2020$ | $10: 45 \mathrm{PM}$ | 5 | 3 | 8 |
| 100 | $8 / 25 / 2020$ | $11: 00 \mathrm{PM}$ | 10 | 6 | 16 |
| 101 | $8 / 25 / 2020$ | $11: 15 \mathrm{PM}$ | 10 | 2 | 12 |
| 102 | $8 / 25 / 2020$ | $11: 30 \mathrm{PM}$ | 6 | 4 | 10 |
| 103 | $8 / 25 / 2020$ | $11: 45 \mathrm{PM}$ | 5 | 2 | 7 |
| 104 |  |  | 1908 | 1913 | 3821 |
|  |  |  |  |  |  |

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @

Arlington, Texas, United States 76013 APARTMENT DRWY
817.265.8968

Site Code:
Start Date: 08/25/2020
Page No: 3

## Turning Movement Peak Hour Data (7:00 AM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | APARTMENT DRWY Westbound |  |  |  |  | DECHMAN DR <br> Northbound |  |  |  |  | Eastbound St. <br> Eastbound |  |  |  |  | $\begin{gathered} \text { Int. } \\ \text { Total } \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total |  |
| 7:00 AM | 0 | 14 | 0 | 0 | 14 | 19 | 0 | 1 | 0 | 20 | 0 | 14 | 1 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 49 |
| 7:15 AM | 0 | 19 | 0 | 0 | 19 | 16 | 0 | 3 | 0 | 19 | 0 | 29 | 3 | 0 | 32 | 0 | 0 | 0 | 0 | 0 | 70 |
| 7:30 AM | 1 | 18 | 0 | 0 | 19 | 15 | 0 | 0 | 0 | 15 | 0 | 21 | 5 | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 60 |
| 7:45 AM | 1 | 16 | 0 | 0 | 17 | 7 | 0 | 1 | 0 | 8 | 0 | 15 | 1 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 41 |
| Total | 2 | 67 | 0 | 0 | 69 | 57 | 0 | 5 | 0 | 62 | 0 | 79 | 10 | 0 | 89 | 0 | 0 | 0 | 0 | 0 | 220 |
| Approach \% | 2.9 | 97.1 | 0.0 | 0.0 | - | 91.9 | 0.0 | 8.1 | 0.0 | - | 0.0 | 88.8 | 11.2 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - |
| Total \% | 0.9 | 30.5 | 0.0 | 0.0 | 31.4 | 25.9 | 0.0 | 2.3 | 0.0 | 28.2 | 0.0 | 35.9 | 4.5 | 0.0 | 40.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | - |
| PHF | 0.500 | 0.882 | 0.000 | 0.000 | 0.908 | 0.750 | 0.000 | 0.417 | 0.000 | 0.775 | 0.000 | 0.681 | 0.500 | 0.000 | 0.695 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.786 |
| Lights | 2 | 67 | 0 | 0 | 69 | 57 | 0 | 5 | 0 | 62 | 0 | 77 | 10 | 0 | 87 | 0 | 0 | 0 | 0 | 0 | 218 |
| \% Lights | 100.0 | 100.0 | - | - | 100.0 | 100.0 | - | 100.0 | - | 100.0 | - | 97.5 | 100.0 | - | 97.8 | - | - | - | - | - | 99.1 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 |
| \% Mediums | 0.0 | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 2.5 | 0.0 | - | 2.2 | - | - | - | - | - | 0.9 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Articulated Trucks | 0.0 | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | - | - | - | - | 0.0 |



Turning Movement Peak Hour Data Plot (7:00 AM)

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @

Arlington, Texas, United States 76013 APARTMENT DRWY
817.265.8968

Site Code:
Start Date: 08/25/2020
Page No: 5

## Turning Movement Peak Hour Data (4:30 PM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | APARTMENT DRWY <br> Westbound |  |  |  |  | DECHMAN DR <br> Northbound |  |  |  |  | Eastbound St. <br> Eastbound |  |  |  |  | $\begin{array}{\|c\|} \hline \text { Int. } \\ \text { Total } \\ \hline \end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total |  |
| 4:30 PM | 4 | 43 | 0 | 0 | 47 | 8 | 0 | 2 | 0 | 10 | 0 | 25 | 16 | 1 | 42 | 0 | 0 | 0 | 0 | 0 | 99 |
| 4:45 PM | 1 | 45 | 0 | 0 | 46 | 13 | 0 | 1 | 0 | 14 | 0 | 32 | 17 | 0 | 49 | 0 | 0 | 0 | 0 | 0 | 109 |
| 5:00 PM | 4 | 47 | 0 | 0 | 51 | 12 | 0 | 2 | 0 | 14 | 0 | 26 | 14 | 0 | 40 | 0 | 0 | 0 | 0 | 0 | 105 |
| 5:15 PM | 3 | 38 | 0 | 0 | 41 | 11 | 0 | 3 | 0 | 14 | 0 | 29 | 21 | 0 | 50 | 0 | 0 | 0 | 0 | 0 | 105 |
| Total | 12 | 173 | 0 | 0 | 185 | 44 | 0 | 8 | 0 | 52 | 0 | 112 | 68 | 1 | 181 | 0 | 0 | 0 | 0 | 0 | 418 |
| Approach \% | 6.5 | 93.5 | 0.0 | 0.0 | - | 84.6 | 0.0 | 15.4 | 0.0 | - | 0.0 | 61.9 | 37.6 | 0.6 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - |
| Total \% | 2.9 | 41.4 | 0.0 | 0.0 | 44.3 | 10.5 | 0.0 | 1.9 | 0.0 | 12.4 | 0.0 | 26.8 | 16.3 | 0.2 | 43.3 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | - |
| PHF | 0.750 | 0.920 | 0.000 | 0.000 | 0.907 | 0.846 | 0.000 | 0.667 | 0.000 | 0.929 | 0.000 | 0.875 | 0.810 | 0.250 | 0.905 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.959 |
| Lights | 12 | 170 | 0 | 0 | 182 | 43 | 0 | 8 | 0 | 51 | 0 | 111 | 68 | 1 | 180 | 0 | 0 | 0 | 0 | 0 | 413 |
| \% Lights | 100.0 | 98.3 | - | - | 98.4 | 97.7 | - | 100.0 | - | 98.1 | - | 99.1 | 100.0 | 100.0 | 99.4 | - | - | - | - | - | 98.8 |
| Mediums | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 5 |
| \% Mediums | 0.0 | 1.7 | - | - | 1.6 | 2.3 | - | 0.0 | - | 1.9 | - | 0.9 | 0.0 | 0.0 | 0.6 | - | - | - | - | - | 1.2 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| \% Articulated Trucks | 0.0 | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - | - | - | - | 0.0 |



Turning Movement Peak Hour Data Plot (4:30 PM)

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @ H-20 EBFR

Arlington, Texas, United States 76013
Site Code:
817.265.8968

Start Date: 08/25/2020
Page No: 3

Turning Movement Peak Hour Data (7:15 AM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | Westbound St. <br> Westbound |  |  |  |  | DECHMAN DR Northbound |  |  |  |  | IH-20 EBFR <br> Eastbound |  |  |  |  | $\begin{gathered} \text { Int. } \\ \text { Total } \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total |  |
| 7:15 AM | 7 | 53 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 101 | 0 | 118 | 16 | 76 | 6 | 0 | 98 | 276 |
| 7:30 AM | 8 | 45 | 0 | 0 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 13 | 112 | 0 | 125 | 27 | 77 | 5 | 0 | 109 | 287 |
| 7:45 AM | 3 | 47 | 0 | 0 | 50 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 71 | 0 | 82 | 16 | 79 | 8 | 0 | 103 | 235 |
| 8:00 AM | 2 | 46 | 0 | 0 | 48 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 68 | 0 | 73 | 16 | 50 | 10 | 0 | 76 | 197 |
| Total | 20 | 191 | 0 | 0 | 211 | 0 | 0 | 0 | 0 | 0 | 0 | 46 | 352 | 0 | 398 | 75 | 282 | 29 | 0 | 386 | 995 |
| Approach \% | 9.5 | 90.5 | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | 11.6 | 88.4 | 0.0 | - | 19.4 | 73.1 | 7.5 | 0.0 | - | - |
| Total \% | 2.0 | 19.2 | 0.0 | 0.0 | 21.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 4.6 | 35.4 | 0.0 | 40.0 | 7.5 | 28.3 | 2.9 | 0.0 | 38.8 | - |
| PHF | 0.625 | 0.901 | 0.000 | 0.000 | 0.879 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.676 | 0.786 | 0.000 | 0.796 | 0.694 | 0.892 | 0.725 | 0.000 | 0.885 | 0.867 |
| Lights | 20 | 190 | 0 | 0 | 210 | 0 | 0 | 0 | 0 | 0 | 0 | 45 | 351 | 0 | 396 | 74 | 281 | 29 | 0 | 384 | 990 |
| \% Lights | 100.0 | 99.5 | - | - | 99.5 | - | - | - | - | - | - | 97.8 | 99.7 | - | 99.5 | 98.7 | 99.6 | 100.0 | - | 99.5 | 99.5 |
| Mediums | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 3 |
| \% Mediums | 0.0 | 0.5 | - | - | 0.5 | - | - | - | - | - | - | 0.0 | 0.3 | - | 0.3 | 0.0 | 0.4 | 0.0 | - | 0.3 | 0.3 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 2 |
| \% Articulated Trucks | 0.0 | 0.0 | - | - | 0.0 | - | - | - | - | - | - | 2.2 | 0.0 | - | 0.3 | 1.3 | 0.0 | 0.0 | - | 0.3 | 0.2 |



Turning Movement Peak Hour Data Plot (7:15 AM)

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @ H-20 EBFR

Arlington, Texas, United States 76013
Site Code:
817.265.8968

Start Date: 08/25/2020
Page No: 5

## Turning Movement Peak Hour Data (5:00 PM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | Westbound St. <br> Westbound |  |  |  |  | DECHMAN DR <br> Northbound |  |  |  |  | IH-20 EBFR <br> Eastbound |  |  |  |  | $\begin{gathered} \text { Int. } \\ \text { Total } \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. Total |  |
| 5:00 PM | 1 | 118 | 0 | 0 | 119 | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 71 | 0 | 85 | 42 | 96 | 15 | 0 | 153 | 357 |
| 5:15 PM | 7 | 124 | 0 | 0 | 131 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 40 | 0 | 57 | 48 | 108 | 16 | 0 | 172 | 360 |
| 5:30 PM | 4 | 120 | 0 | 0 | 124 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 52 | 0 | 69 | 32 | 102 | 22 | 0 | 156 | 349 |
| 5:45 PM | 9 | 110 | 0 | 0 | 119 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 63 | 0 | 82 | 41 | 96 | 11 | 0 | 148 | 349 |
| Total | 21 | 472 | 0 | 0 | 493 | 0 | 0 | 0 | 0 | 0 | 0 | 67 | 226 | 0 | 293 | 163 | 402 | 64 | 0 | 629 | 1415 |
| Approach \% | 4.3 | 95.7 | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | 22.9 | 77.1 | 0.0 | - | 25.9 | 63.9 | 10.2 | 0.0 | - | - |
| Total \% | 1.5 | 33.4 | 0.0 | 0.0 | 34.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 4.7 | 16.0 | 0.0 | 20.7 | 11.5 | 28.4 | 4.5 | 0.0 | 44.5 | - |
| PHF | 0.583 | 0.952 | 0.000 | 0.000 | 0.941 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.882 | 0.796 | 0.000 | 0.862 | 0.849 | 0.931 | 0.727 | 0.000 | 0.914 | 0.983 |
| Lights | 21 | 470 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 | 0 | 66 | 223 | 0 | 289 | 161 | 398 | 64 | 0 | 623 | 1403 |
| \% Lights | 100.0 | 99.6 | - | - | 99.6 | - | - | - | - | - | - | 98.5 | 98.7 | - | 98.6 | 98.8 | 99.0 | 100.0 | - | 99.0 | 99.2 |
| Mediums | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 0 | 3 | 1 | 3 | 0 | 0 | 4 | 9 |
| \% Mediums | 0.0 | 0.4 | - | - | 0.4 | - | - | - | - | - | - | 1.5 | 0.9 | - | 1.0 | 0.6 | 0.7 | 0.0 | - | 0.6 | 0.6 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 0 | 0 | 2 | 3 |
| \% Articulated Trucks | 0.0 | 0.0 | - | - | 0.0 | - | - | - | - | - | - | 0.0 | 0.4 | - | 0.3 | 0.6 | 0.2 | 0.0 | - | 0.3 | 0.2 |



Turning Movement Peak Hour Data Plot (5:00 PM)

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @ H-20 WBFR

Arlington, Texas, United States 76013
Site Code:
817.265.8968

Start Date: 08/25/2020
Page No: 3

Turning Movement Peak Hour Data (7:15 AM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | IH-20 WBFR <br> Westbound |  |  |  |  | DECHMAN DR <br> Northbound |  |  |  |  | Eastbound St. <br> Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total |  |
| 7:15 AM | 0 | 21 | 15 | 0 | 36 | 41 | 57 | 4 | 0 | 102 | 5 | 28 | 0 | 1 | 34 | 0 | 0 | 0 | 0 | 0 | 172 |
| 7:30 AM | 0 | 22 | 9 | 0 | 31 | 30 | 59 | 5 | 0 | 94 | 19 | 25 | 0 | 0 | 44 | 0 | 0 | 0 | 0 | 0 | 169 |
| 7:45 AM | 0 | 17 | 10 | 0 | 27 | 31 | 84 | 5 | 0 | 120 | 13 | 15 | 0 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 175 |
| 8:00 AM | 0 | 11 | 6 | 0 | 17 | 39 | 45 | 2 | 0 | 86 | 11 | 13 | 0 | 0 | 24 | 0 | 0 | 0 | 0 | 0 | 127 |
| Total | 0 | 71 | 40 | 0 | 111 | 141 | 245 | 16 | 0 | 402 | 48 | 81 | 0 | 1 | 130 | 0 | 0 | 0 | 0 | 0 | 643 |
| Approach \% | 0.0 | 64.0 | 36.0 | 0.0 | - | 35.1 | 60.9 | 4.0 | 0.0 | - | 36.9 | 62.3 | 0.0 | 0.8 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - |
| Total \% | 0.0 | 11.0 | 6.2 | 0.0 | 17.3 | 21.9 | 38.1 | 2.5 | 0.0 | 62.5 | 7.5 | 12.6 | 0.0 | 0.2 | 20.2 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | - |
| PHF | 0.000 | 0.807 | 0.667 | 0.000 | 0.771 | 0.860 | 0.729 | 0.800 | 0.000 | 0.838 | 0.632 | 0.723 | 0.000 | 0.250 | 0.739 | 0.000 | 0.000 | 0.000 | 0.000 | 0.000 | 0.919 |
| Lights | 0 | 71 | 40 | 0 | 111 | 140 | 233 | 16 | 0 | 389 | 47 | 81 | 0 | 1 | 129 | 0 | 0 | 0 | 0 | 0 | 629 |
| \% Lights | - | 100.0 | 100.0 | - | 100.0 | 99.3 | 95.1 | 100.0 | - | 96.8 | 97.9 | 100.0 | - | 100.0 | 99.2 | - | - | - | - | - | 97.8 |
| Mediums | 0 | 0 | 0 | 0 | 0 | 1 | 9 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 |
| \% Mediums | - | 0.0 | 0.0 | - | 0.0 | 0.7 | 3.7 | 0.0 | - | 2.5 | 0.0 | 0.0 | - | 0.0 | 0.0 | - | - | - | - | - | 1.6 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 4 |
| \% Articulated Trucks | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 1.2 | 0.0 | - | 0.7 | 2.1 | 0.0 | - | 0.0 | 0.8 | - | - | - | - | - | 0.6 |



Turning Movement Peak Hour Data Plot (7:15 AM)

# GRAM Traffic NTX Inc. <br> 1120 W. Lovers Lane 

Count Name: DECHMAN DR @ IH-20 WBFR

Arlington, Texas, United States 76013
Site Code:
Start Date: 08/25/2020
817.265.8968

Page No: 5

Turning Movement Peak Hour Data (4:45 PM)

| Start Time | DECHMAN DR <br> Southbound |  |  |  |  | IH-20 WBFR <br> Westbound |  |  |  |  | DECHMAN DR <br> Northbound |  |  |  |  | Eastbound St. Eastbound |  |  |  |  | $\begin{aligned} & \text { Int. } \\ & \text { Total } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total | Left | Thru | Right | U-Turn | App. <br> Total |  |
| 4:45 PM | 0 | 36 | 20 | 1 | 57 | 92 | 94 | 7 | 0 | 193 | 21 | 44 | 0 | 1 | 66 | 0 | 1 | 0 | 0 | 1 | 317 |
| 5:00 PM | 0 | 37 | 22 | 0 | 59 | 86 | 105 | 6 | 0 | 197 | 21 | 38 | 0 | 1 | 60 | 0 | 0 | 0 | 0 | 0 | 316 |
| 5:15 PM | 0 | 42 | 13 | 0 | 55 | 94 | 109 | 10 | 0 | 213 | 20 | 50 | 0 | 2 | 72 | 0 | 0 | 0 | 0 | 0 | 340 |
| 5:30 PM | 0 | 26 | 11 | 0 | 37 | 102 | 95 | 10 | 0 | 207 | 18 | 36 | 0 | 0 | 54 | 0 | 0 | 0 | 0 | 0 | 298 |
| Total | 0 | 141 | 66 | 1 | 208 | 374 | 403 | 33 | 0 | 810 | 80 | 168 | 0 | 4 | 252 | 0 | 1 | 0 | 0 | 1 | 1271 |
| Approach \% | 0.0 | 67.8 | 31.7 | 0.5 | - | 46.2 | 49.8 | 4.1 | 0.0 | - | 31.7 | 66.7 | 0.0 | 1.6 | - | 0.0 | 100.0 | 0.0 | 0.0 | - | - |
| Total \% | 0.0 | 11.1 | 5.2 | 0.1 | 16.4 | 29.4 | 31.7 | 2.6 | 0.0 | 63.7 | 6.3 | 13.2 | 0.0 | 0.3 | 19.8 | 0.0 | 0.1 | 0.0 | 0.0 | 0.1 | - |
| PHF | 0.000 | 0.839 | 0.750 | 0.250 | 0.881 | 0.917 | 0.924 | 0.825 | 0.000 | 0.951 | 0.952 | 0.840 | 0.000 | 0.500 | 0.875 | 0.000 | 0.250 | 0.000 | 0.000 | 0.250 | 0.935 |
| Lights | 0 | 139 | 64 | 1 | 204 | 372 | 401 | 32 | 0 | 805 | 79 | 166 | 0 | 4 | 249 | 0 | 0 | 0 | 0 | 0 | 1258 |
| \% Lights | - | 98.6 | 97.0 | 100.0 | 98.1 | 99.5 | 99.5 | 97.0 | - | 99.4 | 98.8 | 98.8 | - | 100.0 | 98.8 | - | 0.0 | - | - | 0.0 | 99.0 |
| Mediums | 0 | 2 | 2 | 0 | 4 | 2 | 2 | 1 | 0 | 5 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 11 |
| \% Mediums | - | 1.4 | 3.0 | 0.0 | 1.9 | 0.5 | 0.5 | 3.0 | - | 0.6 | 0.0 | 0.6 | - | 0.0 | 0.4 | - | 100.0 | - | - | 100.0 | 0.9 |
| Articulated Trucks | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 2 |
| \% Articulated Trucks | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | 1.3 | 0.6 | - | 0.0 | 0.8 | - | 0.0 | - | - | 0.0 | 0.2 |



Turning Movement Peak Hour Data Plot (4:45 PM)

## Cottages at Dechman - Grand Prairie

Vistro File: C:I...।Cottages vistro.vistro
Scenario 1 AM
Report File: C:I...Ivistro am.pdf
9/28/2020

Turning Movement Volume: Detail

| ID | Intersection Name | Volume Type | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |  |
| 1 | Dechman Dr \& access | Final Base | 0 | 79 | 10 | 2 | 67 | 0 | 0 | 0 | 0 | 57 | 0 | 5 | 220 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 27 | 0 | 0 | 0 | 0 | 1 | 4 | 0 | 79 | 0 | 0 | 0 | 111 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 27 | 81 | 10 | 2 | 69 | 1 | 4 | 0 | 79 | 59 | 0 | 5 | 337 |


| ID | Intersection Name | Volume Type | Northbound |  | Southbound |  | Westbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Thru | Right | Left | Thru | Right |  |
| 2 | Dechman Dr \& WB frontage | Final Base | 49 | 81 | 71 | 40 | 141 | 245 | 16 | 643 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 0 | 17 | 29 | 50 | 0 | 0 | 10 | 106 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 50 | 100 | 102 | 91 | 145 | 252 | 26 | 766 |


| ID | Intersection Name | Volume Type | Northbound |  | Southbound |  | Eastbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Thru | Right | Left | Thru | Left | Thru | Right |  |
| 3 | Dechman Dr \& EB frontage rd | Final Base | 46 | 352 | 20 | 191 | 75 | 282 | 29 | 995 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 0 | 0 | 29 | 0 | 17 | 0 | 0 | 46 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 47 | 363 | 50 | 197 | 94 | 290 | 30 | 1071 |

## Cottages at Dechman - Grand Prairie

Vistro File: C:I...ICottages vistro.vistro

## Turning Movement Volume: Detail

| ID | Intersection Name | Volume Type | Northbound |  |  | Southbound |  |  | Eastbound |  |  | Westbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right | Left | Thru | Right |  |
| 1 | Dechman Dr \& access | Final Base | 0 | 112 | 68 | 12 | 173 | 0 | 0 | 0 | 0 | 44 | 0 | 8 | 417 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 90 | 0 | 0 | 0 | 0 | 5 | 3 | 0 | 52 | 0 | 0 | 0 | 150 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 90 | 115 | 70 | 12 | 178 | 5 | 3 | 0 | 52 | 45 | 0 | 8 | 578 |


| ID | Intersection Name | Volume Type | Northbound |  | Southbound |  | Westbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Left | Thru | Thru | Right | Left | Thru | Right |  |
| 2 | Dechman Dr \& WB frontage | Final Base | 84 | 168 | 141 | 66 | 374 | 403 | 33 | 1269 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 0 | 57 | 19 | 33 | 0 | 0 | 33 | 142 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 87 | 230 | 164 | 101 | 385 | 415 | 67 | 1449 |


| ID | Intersection Name | Volume Type | Northbound |  | Southbound |  | Eastbound |  |  | Total Volume |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Thru | Right | Left | Thru | Left | Thru | Right |  |
| 3 | Dechman Dr \& EB frontage rd | Final Base | 67 | 226 | 21 | 472 | 163 | 402 | 64 | 1415 |
|  |  | Growth Factor | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | 1.03 | - |
|  |  | In Process | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Net New Trips | 0 | 0 | 19 | 0 | 57 | 0 | 0 | 76 |
|  |  | Other | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
|  |  | Future Total | 69 | 233 | 41 | 486 | 225 | 414 | 66 | 1534 |

## TRIP GENERATION

| Project Information |  |  |
| :--- | :--- | :--- |
| Project Name: |  |  |
| No: | Grand Prairie Cottages |  |
| Date: | Grand Prairie |  |
| City: | TX |  |
| State/Province: |  |  |
| Zip/Postal Code: |  |  |
| Country: |  |  |
| Client Name: | JBI |  |
| Analyst's Name: | SPI |  |
| Edition: | Trip Gen Manual, 10th Ed |  |


| Land Use | Size | Daily |  | AM |  | PM |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Entry | Exit | Entry | Exit | Entry | Exit |
| 210-Single-Family Detached Housing |  |  |  |  |  |  |  |
| (General Urban/Suburban) | 150 Dwelling Units | 755 | 755 | 28 | 83 | 95 | 55 |
| Reduction |  | 0 | 0 | 0 | 0 |  | 0 |
| Internal |  | 0 | 0 | 0 | 0 |  | 0 |
| Pass-by |  | 0 | 0 | 0 | 0 |  | 0 |
| Non-pass-by |  | 755 | 755 | 28 | 83 | 95 | 55 |
| Total |  | 755 | 755 | 28 | 83 | 95 | 55 |
| Total Reduction |  | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Internal |  | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Pass-by |  | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Non-pass-by |  | 755 | 755 | 28 | 83 | 95 | 55 |

## CAPACITY ANALYSIS

## Existing Conditions

|  | 4 |  |  |  |  |  |  |  |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个 $\uparrow$ | 「 |  |  |  |  | 中t |  | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume（vph） | 75 | 282 | 29 | 0 | 0 | 0 | 0 | 46 | 352 | 20 | 191 | 0 |
| Future Volume（vph） | 75 | 282 | 29 | 0 | 0 | 0 | 0 | 46 | 352 | 20 | 191 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 200 |  | 200 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Satd．Flow（prot） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3123 | 0 | 1805 | 1900 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.488 |  |  |
| Satd．Flow（perm） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3123 | 0 | 927 | 1900 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 55 |  |  |  |  | 400 |  |  |  |  |
| Link Speed（mph） |  | 40 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 199 |  |  | 262 |  |  | 374 |  |  | 525 |  |
| Travel Time（s） |  | 3.4 |  |  | 6.0 |  |  | 8.5 |  |  | 11.9 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.90 | 0.90 | 0.90 | 0.90 | 0.87 | 0.87 | 0.87 | 0.87 | 0.90 |
| Heavy Vehicles（\％） | 1\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 0\％ | 0\％ |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 86 | 324 | 33 | 0 | 0 | 0 | 0 | 458 | 0 | 23 | 220 | 0 |
| Turn Type | Perm | NA | Perm |  |  |  |  | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  |  |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  | 4 |  |  |  |  |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 4 |  |  |  |  | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 20.0 | 20.0 | 20.0 |  |  |  |  | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split（s） | 26.0 | 26.0 | 26.0 |  |  |  |  | 16.0 |  | 16.0 | 16.0 |  |
| Total Split（s） | 31.0 | 31.0 | 31.0 |  |  |  |  | 29.0 |  | 29.0 | 29.0 |  |
| Total Split（\％） | 51．7\％ | 51．7\％ | 51．7\％ |  |  |  |  | 48．3\％ |  | 48．3\％ | 48．3\％ |  |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 |  |  |  |  | 4.0 |  | 4.0 | 4.0 |  |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 |  |  |  |  | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time（s） | 6.0 | 6.0 | 6.0 |  |  |  |  | 6.0 |  | 6.0 | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None |  |  |  |  | Min |  | Min | Min |  |
| Act Effct Green（s） | 20.0 | 20.0 | 20.0 |  |  |  |  | 11.0 |  | 11.0 | 11.0 |  |
| Actuated g／C Ratio | 0.47 | 0.47 | 0.47 |  |  |  |  | 0.26 |  | 0.26 | 0.26 |  |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.10 | 0.19 | 0.04 |  |  |  |  | 0.42 |  | 0.10 | 0.45 |  |
| Control Delay | 7.4 | 7.4 | 1.9 |  |  |  |  | 4.0 |  | 13.2 | 16.8 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 7.4 | 7.4 | 1.9 |  |  |  |  | 4.0 |  | 13.2 | 16.8 |  |
| LOS | A | A | A |  |  |  |  | A |  | B | B |  |
| Approach Delay |  | 7.0 |  |  |  |  |  | 4.0 |  |  | 16.5 |  |
| Approach LOS |  | A |  |  |  |  |  | A |  |  | B |  |
| Queue Length 50th（ft） | 10 | 21 | 0 |  |  |  |  | 5 |  | 4 | 45 |  |
| Queue Length 95th（ft） | 29 | 42 | 7 |  |  |  |  | 27 |  | 16 | 86 |  |
| Internal Link Dist（ft） |  | 119 |  |  | 182 |  |  | 294 |  |  | 445 |  |
| Turn Bay Length（ft） | 200 |  | 200 |  |  |  |  |  |  |  |  |  |
| Base Capacity（vph） | 1039 | 2099 | 962 |  |  |  |  | 1857 |  | 496 | 1016 |  |


|  | 4 |  |  | $\dagger$ |  | 4 | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.08 | 0.15 | 0.03 |  |  |  |  | 0.25 |  | 0.05 | 0.22 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 60 |  |  |  |  |  |  |  |  |  |  |  |  |

Actuated Cycle Length: 43
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.45
Intersection Signal Delay: 7.8 Intersection LOS: A
Intersection Capacity Utilization 52.7\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 3: Dechman Dr \& EB frontage


|  | 4 |  |  |  |  |  | 4 |  |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  |  |  | ＊ | 个 $\uparrow$ | 「 | \％ | $\uparrow$ |  |  | 个 ${ }^{2}$ |  |
| Traffic Volume（vph） | 0 | 0 | 0 | 141 | 245 | 16 | 49 | 81 | 0 | 0 | 71 | 40 |
| Future Volume（vph） | 0 | 0 | 0 | 141 | 245 | 16 | 49 | 81 | 0 | 0 | 71 | 40 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 0 |  | 0 | 200 |  | 200 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Satd．Flow（prot） | 0 | 0 | 0 | 1805 | 3574 | 1615 | 1770 | 1900 | 0 | 0 | 3415 | 0 |
| Flt Permitted |  |  |  | 0.950 |  |  | 0.676 |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 0 | 0 | 1805 | 3574 | 1615 | 1259 | 1900 | 0 | 0 | 3415 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  |  |  |  | 55 |  |  |  |  | 43 |  |
| Link Speed（mph） |  | 30 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 143 |  |  | 189 |  |  | 525 |  |  | 423 |  |
| Travel Time（s） |  | 3.3 |  |  | 3.2 |  |  | 11.9 |  |  | 9.6 |  |
| Peak Hour Factor | 0.90 | 0.90 | 0.90 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.90 | 0.90 | 0.92 | 0.92 |
| Heavy Vehicles（\％） | 0\％ | 0\％ | 0\％ | 0\％ | 1\％ | 0\％ | 2\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 0 | 0 | 153 | 266 | 17 | 53 | 88 | 0 | 0 | 120 | 0 |
| Turn Type |  |  |  | Perm | NA | Perm | Perm | NA |  |  | NA |  |
| Protected Phases |  |  |  |  | 8 |  |  | 2 |  |  | 6 |  |
| Permitted Phases |  |  |  | 8 |  | 8 | 2 |  |  |  |  |  |
| Detector Phase |  |  |  | 8 | 8 | 8 | 2 | 2 |  |  | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） |  |  |  | 20.0 | 20.0 | 20.0 | 10.0 | 10.0 |  |  | 10.0 |  |
| Minimum Split（s） |  |  |  | 26.0 | 26.0 | 26.0 | 16.0 | 16.0 |  |  | 16.0 |  |
| Total Split（s） |  |  |  | 35.0 | 35.0 | 35.0 | 25.0 | 25.0 |  |  | 25.0 |  |
| Total Split（\％） |  |  |  | 58．3\％ | 58．3\％ | 58．3\％ | 41．7\％ | 41．7\％ |  |  | 41．7\％ |  |
| Yellow Time（s） |  |  |  | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |
| All－Red Time（s） |  |  |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  |  | 2.0 |  |
| Lost Time Adjust（s） |  |  |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  | 0.0 |  |
| Total Lost Time（s） |  |  |  | 6.0 | 6.0 | 6.0 | 6.0 | 6.0 |  |  | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode |  |  |  | None | None | None | Min | Min |  |  | Min |  |
| Act Effct Green（s） |  |  |  | 20.0 | 20.0 | 20.0 | 10.2 | 10.2 |  |  | 10.2 |  |
| Actuated g／C Ratio |  |  |  | 0.47 | 0.47 | 0.47 | 0.24 | 0.24 |  |  | 0.24 |  |
| v／c Ratio |  |  |  | 0.18 | 0.16 | 0.02 | 0.17 | 0.19 |  |  | 0.14 |  |
| Control Delay |  |  |  | 7.0 | 6.6 | 0.6 | 14.5 | 14.1 |  |  | 9.5 |  |
| Queue Delay |  |  |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  | 0.0 |  |
| Total Delay |  |  |  | 7.0 | 6.6 | 0.6 | 14.5 | 14.1 |  |  | 9.5 |  |
| LOS |  |  |  | A | A | A | B | B |  |  | A |  |
| Approach Delay |  |  |  |  | 6.5 |  |  | 14.3 |  |  | 9.5 |  |
| Approach LOS |  |  |  |  | A |  |  | B |  |  | A |  |
| Queue Length 50th（ft） |  |  |  | 19 | 16 | 0 | 10 | 17 |  |  | 7 |  |
| Queue Length 95th（ft） |  |  |  | 41 | 31 | 2 | 30 | 42 |  |  | 21 |  |
| Internal Link Dist（ft） |  | 63 |  |  | 109 |  |  | 445 |  |  | 343 |  |
| Turn Bay Length（ft） |  |  |  | 200 |  | 200 |  |  |  |  |  |  |
| Base Capacity（vph） |  |  |  | 1240 | 2454 | 1126 | 566 | 855 |  |  | 1560 |  |


|  | $\stackrel{ }{*}$ | $\rightarrow$ |  | 7 |  | 4 | 4 | $\uparrow$ | 7 | $\downarrow$ | ¢ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Spillback Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Storage Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Reduced v/c Ratio |  |  |  | 0.12 | 0.11 | 0.02 | 0.09 | 0.10 |  |  | 0.08 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60
Actuated Cycle Length: 42.2
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.19
Intersection Signal Delay: 8.6 Intersection LOS: A
Intersection Capacity Utilization 52.7\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 6: Dechman Dr \& WB frontage


|  | $\checkmark$ | 4 | $\dagger$ | 7 | $\checkmark$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | $\uparrow$ |  | \% | $\uparrow$ |
| Traffic Volume (vph) | 57 | 5 | 79 | 10 | 2 | 67 |
| Future Volume (vph) | 57 | 5 | 79 | 10 | 2 | 67 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 | 0 |  | 0 | 200 |  |
| Storage Lanes | 1 | 0 |  | 0 | 1 |  |
| Taper Length (ft) | 100 |  |  |  | 100 |  |
| Satd. Flow (prot) | 1798 | 0 | 1870 | 0 | 1805 | 1900 |
| Flt Permitted | 0.956 |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 1798 | 0 | 1870 | 0 | 1805 | 1900 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 153 |  | 423 |  |  | 289 |
| Travel Time (s) | 3.5 |  | 9.6 |  |  | 6.6 |
| Peak Hour Factor | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 | 0.79 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 78 | 0 | 113 | 0 | 3 | 85 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: | ther |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 14.9\% |  |  |  | ICU Level of Service A |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |


|  | 4 |  |  |  |  |  |  |  |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 个 $\uparrow$ | 「 |  |  |  |  | 中t |  | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Volume（vph） | 163 | 402 | 64 | 0 | 0 | 0 | 0 | 67 | 226 | 21 | 472 | 0 |
| Future Volume（vph） | 163 | 402 | 64 | 0 | 0 | 0 | 0 | 67 | 226 | 21 | 472 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 200 |  | 200 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Satd．Flow（prot） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3177 | 0 | 1805 | 1900 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.549 |  |  |
| Satd．Flow（perm） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3177 | 0 | 1043 | 1900 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 74 |  |  |  |  | 177 |  |  |  |  |
| Link Speed（mph） |  | 40 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 199 |  |  | 262 |  |  | 374 |  |  | 525 |  |
| Travel Time（s） |  | 3.4 |  |  | 6.0 |  |  | 8.5 |  |  | 11.9 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.90 | 0.90 | 0.90 | 0.90 | 0.87 | 0.87 | 0.87 | 0.87 | 0.90 |
| Heavy Vehicles（\％） | 1\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 0\％ | 0\％ |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 187 | 462 | 74 | 0 | 0 | 0 | 0 | 337 | 0 | 24 | 543 | 0 |
| Turn Type | Perm | NA | Perm |  |  |  |  | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  |  |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  | 4 |  |  |  |  |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 4 |  |  |  |  | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 20.0 | 20.0 | 20.0 |  |  |  |  | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split（s） | 26.0 | 26.0 | 26.0 |  |  |  |  | 16.0 |  | 16.0 | 16.0 |  |
| Total Split（s） | 26.0 | 26.0 | 26.0 |  |  |  |  | 34.0 |  | 34.0 | 34.0 |  |
| Total Split（\％） | 43．3\％ | 43．3\％ | 43．3\％ |  |  |  |  | 56．7\％ |  | 56．7\％ | 56．7\％ |  |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 |  |  |  |  | 4.0 |  | 4.0 | 4.0 |  |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 |  |  |  |  | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time（s） | 6.0 | 6.0 | 6.0 |  |  |  |  | 6.0 |  | 6.0 | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None |  |  |  |  | Min |  | Min | Min |  |
| Act Effct Green（s） | 20.2 | 20.2 | 20.2 |  |  |  |  | 19.9 |  | 19.9 | 19.9 |  |
| Actuated g／C Ratio | 0.39 | 0.39 | 0.39 |  |  |  |  | 0.38 |  | 0.38 | 0.38 |  |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.27 | 0.33 | 0.11 |  |  |  |  | 0.26 |  | 0.06 | 0.75 |  |
| Control Delay | 14.2 | 13.4 | 4.7 |  |  |  |  | 5.4 |  | 9.7 | 20.9 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 14.2 | 13.4 | 4.7 |  |  |  |  | 5.4 |  | 9.7 | 20.9 |  |
| LOS | B | B | A |  |  |  |  | A |  | A | C |  |
| Approach Delay |  | 12.7 |  |  |  |  |  | 5.4 |  |  | 20.5 |  |
| Approach LOS |  | B |  |  |  |  |  | A |  |  | C |  |
| Queue Length 50th（ft） | 38 | 51 | 0 |  |  |  |  | 16 |  | 4 | 137 |  |
| Queue Length 95th（ft） | 90 | 96 | 22 |  |  |  |  | 33 |  | 15 | 215 |  |
| Internal Link Dist（ft） |  | 119 |  |  | 182 |  |  | 294 |  |  | 445 |  |
| Turn Bay Length（ft） | 200 |  | 200 |  |  |  |  |  |  |  |  |  |
| Base Capacity（vph） | 691 | 1397 | 670 |  |  |  |  | 1803 |  | 565 | 1029 |  |


|  | 4 | $\rightarrow$ |  | $\downarrow$ |  | 4 | 4 | $\uparrow$ | $>$ | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.27 | 0.33 | 0.11 |  |  |  |  | 0.19 |  | 0.04 | 0.53 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60
Actuated Cycle Length: 52.3
Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.75
Intersection Signal Delay: 13.9
Intersection LOS: B
Intersection Capacity Utilization 52.4\%
ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 3: Dechman Dr \& EB frontage


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 | $\rightarrow$ |  | $t$ |  | 4 | 4 | 4 | $>$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Spillback Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Storage Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Reduced v/c Ratio |  |  |  | 0.33 | 0.18 | 0.03 | 0.20 | 0.24 |  |  | 0.16 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60
Actuated Cycle Length: 44.7
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.47
Intersection Signal Delay: 10.6 Intersection LOS: B
Intersection Capacity Utilization 52.4\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 6: Dechman Dr \& WB frontage



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





Full Build 2021 Conditions

|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | 4 | $\rightarrow$ | 7 | $\dagger$ |  |  | 4 | $\uparrow$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.10 | 0.16 | 0.04 |  |  |  |  | 0.26 |  | 0.12 | 0.22 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60
Actuated Cycle Length: 43.3
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.46
Intersection Signal Delay: 8.3 Intersection LOS: A
Intersection Capacity Utilization 53.1\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 3: Dechman Dr \& EB frontage


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\rangle$ | $\rightarrow$ |  | $\dagger$ | $\leftarrow$ | 4 | 4 | $\uparrow$ | $>$ | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Spillback Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Storage Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Reduced v/c Ratio |  |  |  | 0.13 | 0.11 | 0.02 | 0.10 | 0.13 |  |  | 0.13 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60
Actuated Cycle Length: 42
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.24
Intersection Signal Delay: 8.5 Intersection LOS: A
Intersection Capacity Utilization 53.1\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 6: Dechman Dr \& WB frontage



|  | 4 |  |  |  |  |  |  |  |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个个 | ${ }^{*}$ |  |  |  |  | 性 |  | ${ }^{4}$ | $\uparrow$ |  |
| Traffic Volume（vph） | 225 | 414 | 66 | 0 | 0 | 0 | 0 | 69 | 233 | 41 | 486 | 0 |
| Future Volume（vph） | 225 | 414 | 66 | 0 | 0 | 0 | 0 | 69 | 233 | 41 | 486 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 200 |  | 200 | 0 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Satd．Flow（prot） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3177 | 0 | 1805 | 1900 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.544 |  |  |
| Satd．Flow（perm） | 1787 | 3610 | 1615 | 0 | 0 | 0 | 0 | 3177 | 0 | 1034 | 1900 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 76 |  |  |  |  | 167 |  |  |  |  |
| Link Speed（mph） |  | 40 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 199 |  |  | 262 |  |  | 374 |  |  | 525 |  |
| Travel Time（s） |  | 3.4 |  |  | 6.0 |  |  | 8.5 |  |  | 11.9 |  |
| Peak Hour Factor | 0.87 | 0.87 | 0.87 | 0.90 | 0.90 | 0.90 | 0.90 | 0.87 | 0.87 | 0.87 | 0.87 | 0.90 |
| Heavy Vehicles（\％） | 1\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 0\％ | 2\％ | 0\％ | 0\％ | 0\％ | 0\％ |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 259 | 476 | 76 | 0 | 0 | 0 | 0 | 347 | 0 | 47 | 559 | 0 |
| Turn Type | Perm | NA | Perm |  |  |  |  | NA |  | Perm | NA |  |
| Protected Phases |  | 4 |  |  |  |  |  | 2 |  |  | 6 |  |
| Permitted Phases | 4 |  | 4 |  |  |  |  |  |  | 6 |  |  |
| Detector Phase | 4 | 4 | 4 |  |  |  |  | 2 |  | 6 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 20.0 | 20.0 | 20.0 |  |  |  |  | 10.0 |  | 10.0 | 10.0 |  |
| Minimum Split（s） | 26.0 | 26.0 | 26.0 |  |  |  |  | 16.0 |  | 16.0 | 16.0 |  |
| Total Split（s） | 26.0 | 26.0 | 26.0 |  |  |  |  | 34.0 |  | 34.0 | 34.0 |  |
| Total Split（\％） | 43．3\％ | 43．3\％ | 43．3\％ |  |  |  |  | 56．7\％ |  | 56．7\％ | 56．7\％ |  |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 |  |  |  |  | 4.0 |  | 4.0 | 4.0 |  |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 |  |  |  |  | 2.0 |  | 2.0 | 2.0 |  |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Lost Time（s） | 6.0 | 6.0 | 6.0 |  |  |  |  | 6.0 |  | 6.0 | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  |  |  |  |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  |  |  |  |  |  |
| Recall Mode | None | None | None |  |  |  |  | Min |  | Min | Min |  |
| Act Effct Green（s） | 20.2 | 20.2 | 20.2 |  |  |  |  | 20.5 |  | 20.5 | 20.5 |  |
| Actuated g／C Ratio | 0.38 | 0.38 | 0.38 |  |  |  |  | 0.39 |  | 0.39 | 0.39 |  |
| v／c Ratio | 0.38 | 0.34 | 0.11 |  |  |  |  | 0.26 |  | 0.12 | 0.76 |  |
| Control Delay | 15.5 | 13.8 | 4.7 |  |  |  |  | 5.7 |  | 10.3 | 21.1 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  |  |  |  | 0.0 |  | 0.0 | 0.0 |  |
| Total Delay | 15.5 | 13.8 | 4.7 |  |  |  |  | 5.7 |  | 10.3 | 21.1 |  |
| LOS | B | B | A |  |  |  |  | A |  | B | C |  |
| Approach Delay |  | 13.5 |  |  |  |  |  | 5.7 |  |  | 20.3 |  |
| Approach LOS |  | B |  |  |  |  |  | A |  |  | C |  |
| Queue Length 50th（ft） | 56 | 53 | 0 |  |  |  |  | 17 |  | 9 | 143 |  |
| Queue Length 95th（ft） | 123 | 99 | 22 |  |  |  |  | 36 |  | 24 | 223 |  |
| Internal Link Dist（ft） |  | 119 |  |  | 182 |  |  | 294 |  |  | 445 |  |
| Turn Bay Length（ft） | 200 |  | 200 |  |  |  |  |  |  |  |  |  |
| Base Capacity（vph） | 683 | 1380 | 664 |  |  |  |  | 1778 |  | 553 | 1017 |  |


|  | 4 | $\rightarrow$ | $\geqslant$ | $\checkmark$ |  | 4 | 4 | $\dagger$ | 7 | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Spillback Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Storage Cap Reductn | 0 | 0 | 0 |  |  |  |  | 0 |  | 0 | 0 |  |
| Reduced v/c Ratio | 0.38 | 0.34 | 0.11 |  |  |  |  | 0.20 |  | 0.08 | 0.55 |  |

## Intersection Summary

```
Area Type: Other
```

Cycle Length: 60

Actuated Cycle Length: 52.9
Natural Cycle: 55
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.76
Intersection Signal Delay: 14.3
Intersection LOS: B
Intersection Capacity Utilization 53.0\%
ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 3: Dechman Dr \& EB frontage


|  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


|  | $\Rightarrow$ |  |  | $\checkmark$ |  | 4 | 4 | $\dagger$ | $>$ | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Starvation Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Spillback Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Storage Cap Reductn |  |  |  | 0 | 0 | 0 | 0 | 0 |  |  | 0 |  |
| Reduced v/c Ratio |  |  |  | 0.35 | 0.19 | 0.07 | 0.23 | 0.33 |  |  | 0.20 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 60 |  |  |  |  |  |  |  |  |  |  |  |  |

Actuated Cycle Length: 46.6
Natural Cycle: 45
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.50
Intersection Signal Delay: 11.3 Intersection LOS: B
Intersection Capacity Utilization 53.0\% ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 6: Dechman Dr \& WB frontage







## TURN LANE ANALYSIS

Figure 2-5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.
2-lane roadway (English)
INPUT

| Variable | Value |
| :--- | :---: |
| $85^{\text {th }}$ percentile speed, mph: | 35 |
| Percent of left-turns in advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right), \%:$ | $44 \%$ |
| Advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right)$, veh $/ \mathrm{h}:$ | 203 |
| Opposing volume $\left(\mathrm{V}_{\mathrm{O}}\right)$, veh $/ \mathrm{h}:$ | 175 |

OUTPUT

| Variable |  |
| :--- | :---: |
| Limiting advancing volume $\left(\mathrm{V}_{\mathrm{A}}\right)$, veh/h: | Value |
| Guidance for determining the need for a major-road left-turn bay: |  |
| Left-turn treatment warranted. |  |


CALIBRATION CONSTANTS

| Variable | Value |
| :--- | :---: |
| Average time for making left-turn, s: | 4.3 |
| Critical headway, s: | 5.5 |
| Average time for left-turn vehicle to clear the advancing lane, s: | 3.2 |

Figure 2-6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlle
INPUT

| Roadway geometry: | 2-lane roadway |  |
| :--- | :---: | :---: |
| Variable |  | Value |
| Major-road speed, mph: | 35 |  |
| Major-road volume (one direction), veh/h: | 175 |  |
| Right-turn volume, veh/h: | 5 |  |

OUTPUT

| Variable | Value |
| :--- | :---: |
| Limiting right-turn volume, veh/h: | 9348 |
| Guidance for determining the need for a major-road <br> right-turn bay for a 2-lane roadway: |  |
| Do NOT add right-turn bay. |  |



Legislation Details (With Text)

| File \#: | 20-10468 | Version: 1 | Name: | Liquor Stores |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Type: | Agenda Item |  | Status: | Agenda Read |  |
| File created: | 10/7/2020 |  | In control: | Planning |  |
| On agenda: | 10/13/2020 |  | Final action: |  |  |
| Title: | Liquor Stores Update - Presented by Bill Hills, Deputy City Manager, and Rashad Jackson, Planning and Development Director |  |  |  |  |
| Sponsors: |  |  |  |  |  |
| Indexes: |  |  |  |  |  |
| Code sections |  |  |  |  |  |
| Attachments: |  |  |  |  |  |
| Date | Ver. Action By |  |  |  | Result |

Title
Liquor Stores Update - Presented by Bill Hills, Deputy City Manager, and Rashad Jackson, Planning and Development Director

City of Grand Prairie

Legislation Details (With Text)

| File \#: | 20-10391 | Version: 1 | Name: | Community Revitalization Update |
| :---: | :---: | :---: | :---: | :---: |
| Type: | Presentation |  | Status: | Agenda Ready - Committee |
| File created: | 9/16/2020 |  | In control: | City Council Development Committee |
| On agenda: | 10/13/2020 |  | Final action: |  |
| Title: | Community Revitalization Update - Presented by Andrew Fortune, Assistant to the City Manager |  |  |  |
| Sponsors: |  |  |  |  |
| Indexes: |  |  |  |  |
| Code sections: |  |  |  |  |
| Attachments: |  |  |  |  |
| Date | Ver. Action By |  |  | n Result |

Title
Community Revitalization Update - Presented by Andrew Fortune, Assistant to the City Manager


[^0]:    Chairman Jeff Wooldridge

